



Coordination of Freeway Ramp Meters and Arterial Traffic Signals (FOT)

End of Project Presentation

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Outlines

- **Site Selection**
- **Data Collection for Modeling**
- **Modeling and Simulation**
- **Freeway Onramp Metering**
- **Intersection Signal Control**
- **Coordination Strategy**
- **System Integration**
- **Test Results**
- **Next Phase of the Project**



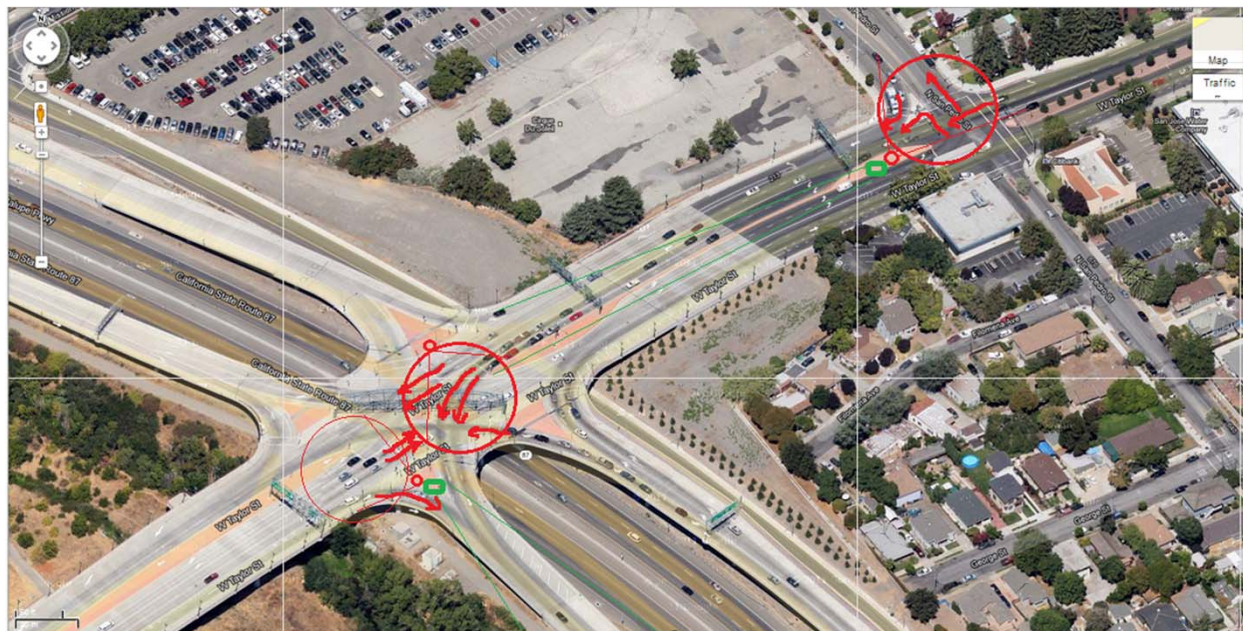
Site Selection

- **Site Selection criteria development**
 - Road geometry, traffic situation, sensor detection
 - Data system
 - Communication systems
 - Traffic controllers (freeway & arterial), 170 → 2070
 - Traffic control software and interface capability
 - Institutional issues
 - Small and relatively independent
- **Data collection for site selection**
 - 5 sites considered: I280-Saratoga, SR87-Taylor, SR85-Camden, I280-Lawrence, SR101-DeLaCruz
 - SR87-Taylor was selected with the project panel



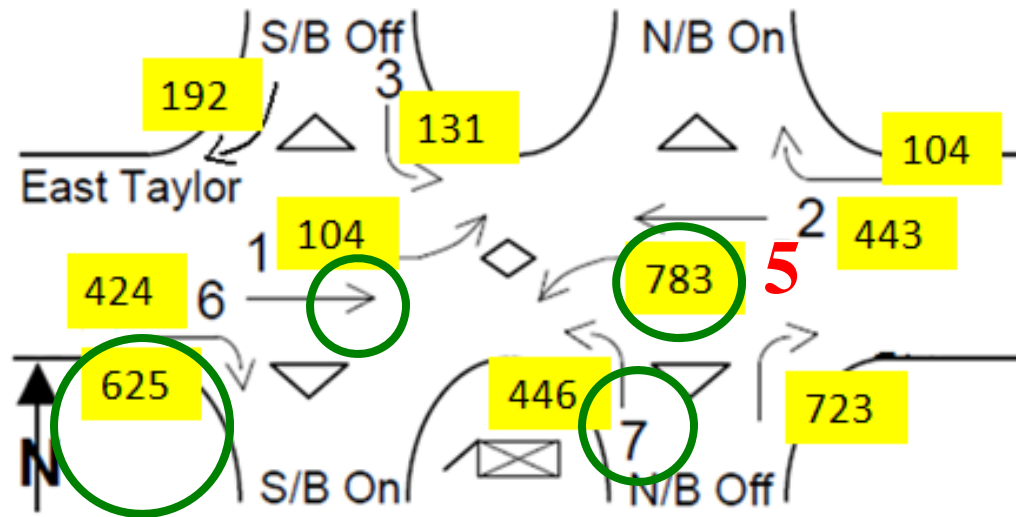
Data Collection for Modeling

- 3 Miovision VCU systems + 3 Video cameras





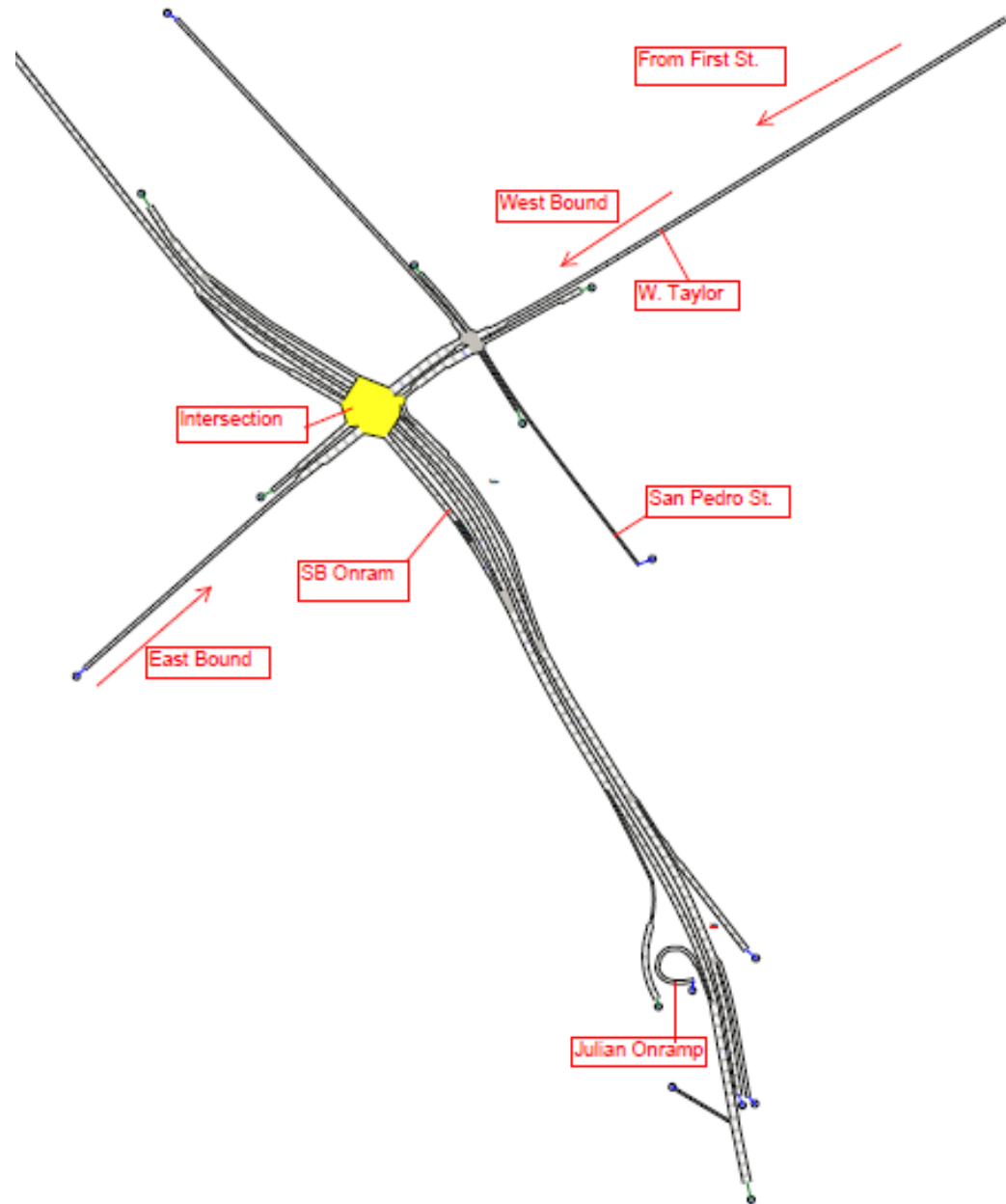
Intersection Traffic Situation



- The Problem
 - EB RT green = Phase 6 Thru green + Phase 7 green → too high priority to access onramp all the time
 - EB Phase 5 LT has large demand, but less priority to access onramp in PM Peak → queue spills over to San Pedros



Modeling and Simulation - Model Calibration





Modeling and Simulation - Model Calibration

- **Data:**
 - Aggregated over 10min
- **Flow**
 - Compute the percentage of acceptable simulated flow (within acceptable error)
 - Aggregated flow in 10min
 - Freeway: detector at upstream of Taylor SB on-ramp, 1 lane available
 - Intersection: 8 movements
 - FHWA recommended flow calibration criteria
- **Speed**
 - Compute the percentage of acceptable simulated speed (speed error less than 5mph)



Calibration Results

- **Intersection:**
 - **EB RT Movement**
 - **Percent of flow values within acceptable error:** mean 92.1%, deviation 6.2%
 - **Percent of flow values with GEH<5:** mean 97.1%, deviation 3.9%
 - **WB LT Movement**
 - **Percent of flow values within acceptable error:** mean 89.2%, deviation 5.8%
 - **Percent of flow values with GEH<5:** mean 92.1%, deviation 4.2%
- **Freeway Mainline:**
 - **Flow**
 - **Percent of flow values within acceptable error:** mean 93.4%, deviation 1.4%
 - **Percent of flow values with GEH<5:** mean 90.3%, deviation 3.1%
 - **Speed**
 - **Percent of speed values with error<5mph:** mean 89.7%, 3.1%



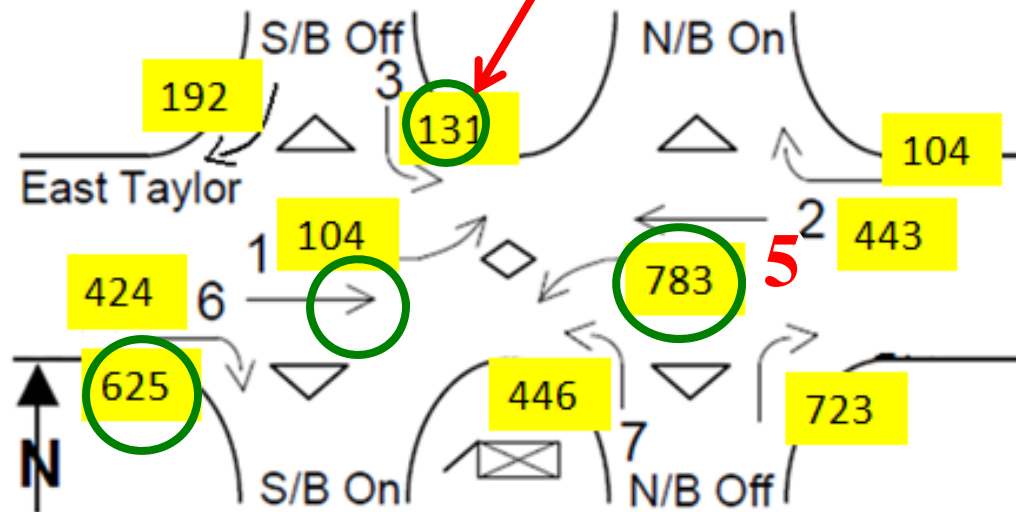
Freeway Onramp Metering

- Ramp metering algorithm
 - ALINEA, control based on occupancy
 - Requiring detectors installed at merging area
- Simulated and Implemented UP ALINEA
 - Modified ALINEA
 - Need to estimate occupancy from upstream measurement.



Intersection Signal Control

Only controls **green time of phase 3**





Intersection Signal Control

- **Initial development:**
 - Optimization for balancing demand and supplies of all major movements and onramp
- **Simplified Strategy for Field Implementation and Test**
 - Keep the current control strategy: actuated control.
 - Limit EB RT Green to give more space at onramp to WB LT
 - Give more spaces at onramp to WB LT to improve throughput and to avoid queue spillovers
 - Change the overlap of EB right-turn, connect EB RT to phase 3 (over lap 6&7 → 6&3).
 - Regulate maximum green of phase 3 to indirectly change green length of EB RT.
 - Regulation algorithm is a queue-overwrite-like design based on occupancy instead of direct queue estimation



Coordination Strategy

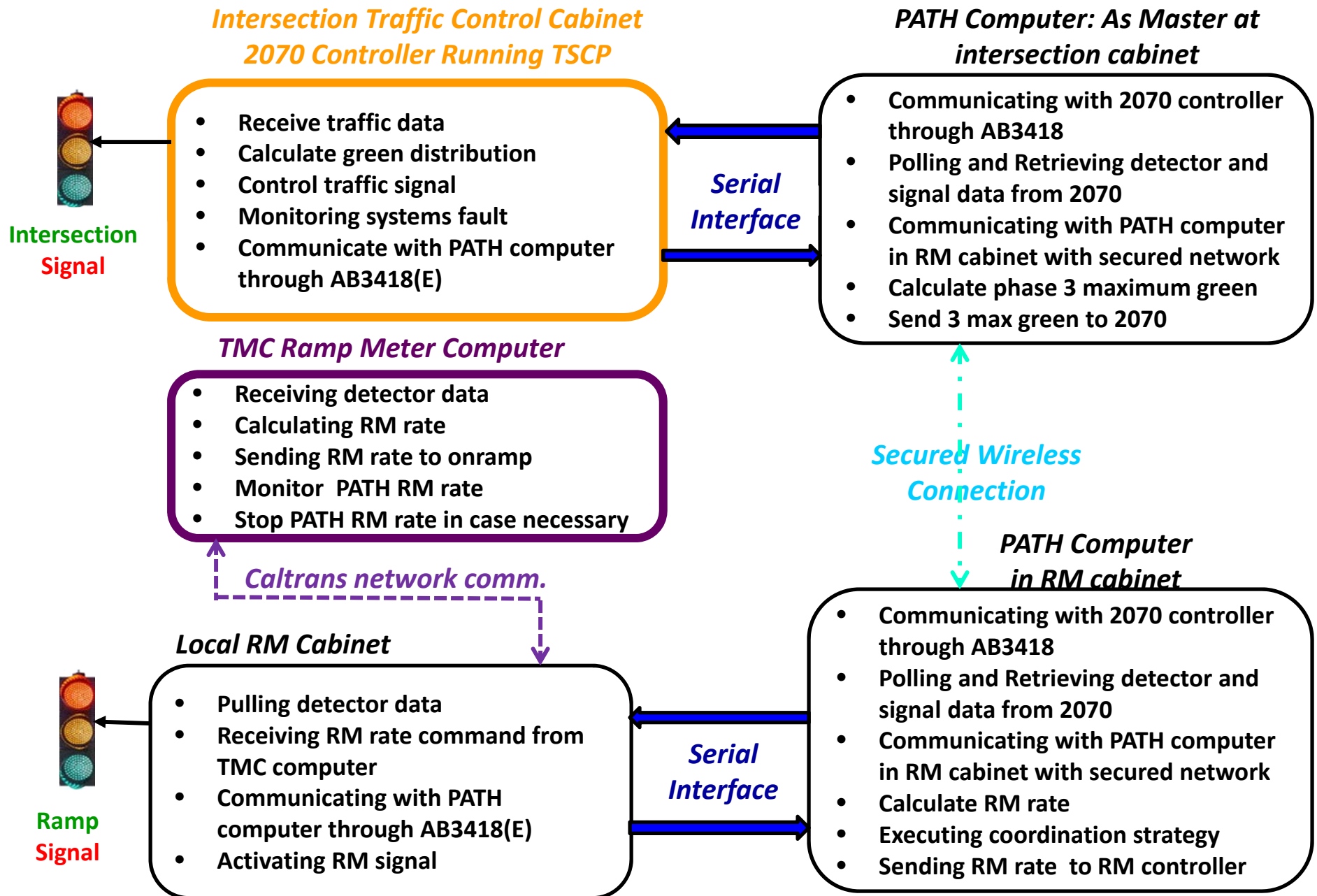
- **Coordination logistics (dependencies)**

**Based on freeway traffic situation (occupancy measurement)
determines RM rate (RM algorithm)**

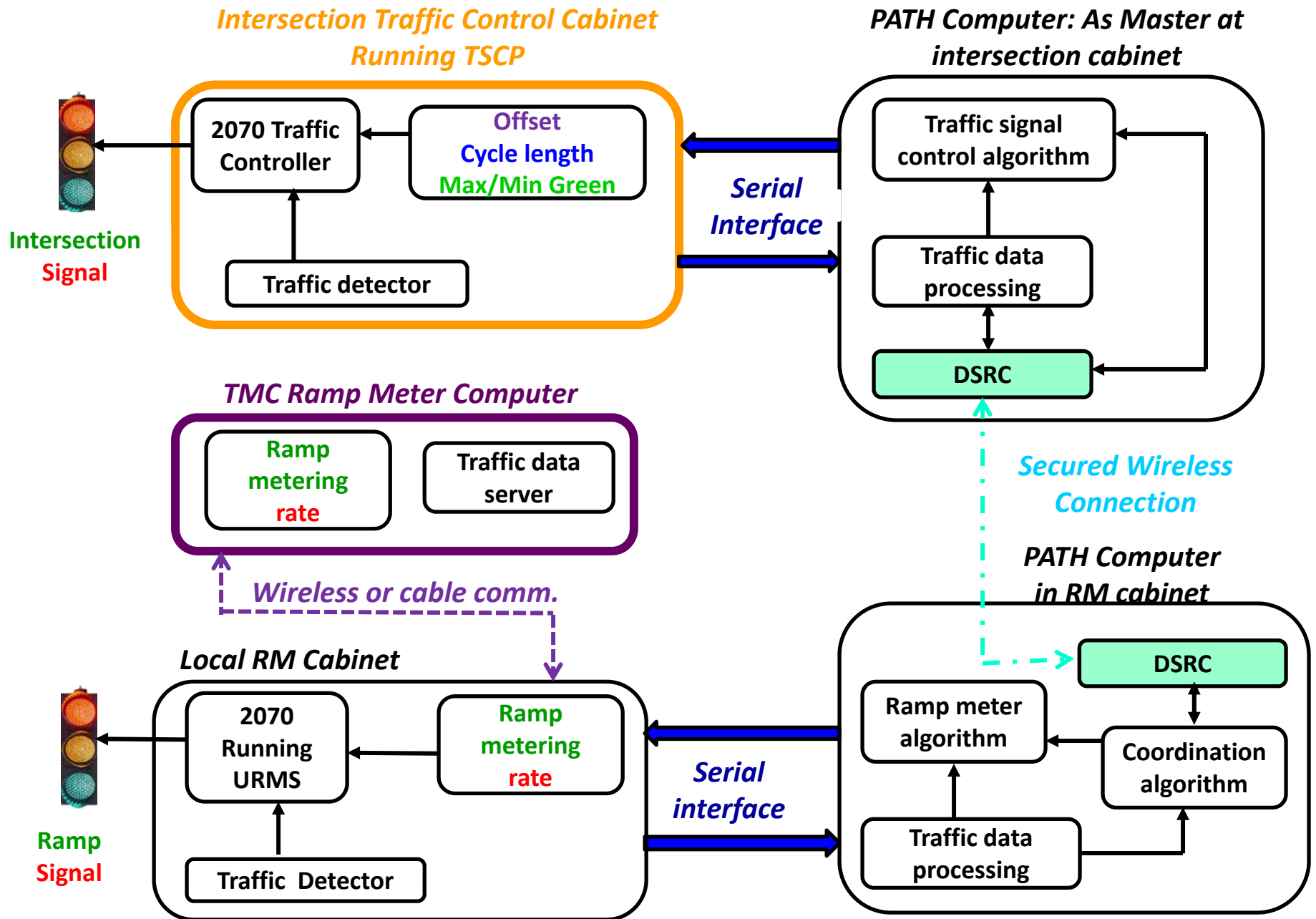
→ estimate the condition of onramp queue

**→ how to distribute the available space between Right Turn
(overlap with Phase 6&3) and Phase 5 Left turn (intersection
traffic control algorithm)**

Functional Diagram



Data Flow Diagram





Test Results – Compare “before” & “after”

- Relevant Detector Locations
- Data Validation
- Focusing on WB LT and EB RT, Onramp and Freeway
- Data Analysis (10 min aggregation)
- Flow Comparison at WB LT Stop Line
- Vehicle Counts of EB RT & WB LT A-Loop
- Occupancy of EB RT & WB LT A-Loop
- Compare Average Across all Effective Days
- Net Benefit at Taylor Intersection
 - Net flow increase
 - Net time delay decrease
- Benefit at Onramp: Accumulated Passage detector Flow
- Affect to Freeway Upstream



Relevant Detector Locations





Test Results

- **Data Validation: Test dates of PATH controller at SR87 Onramp and Taylor Intersection:**
 - PATH control on date: 10/29/13- 11/08/13 (Green)
 - Control, Loop & Overlap changed on date: 11/12/13 (12:30pm)
 - PATH control off for data collection: 11/09/13 – 11/18/13 (Red)
 - Health check conducted; weekend were dropped (Yellow)

PATH Control on date	PATH Control on date	Data Collection Date	Day of the Week	Comments
10/29	11/05	11/12	Tue	
10/30	11/06	11/13	Wed	
10/31	11/07	11/14	Thu	
11/01	11/08	11/15	Fri	
11/02		11/16	Sat	Weekend dropped
11/03		11/17	Sun	Weekend dropped
11/04		11/18	Mon	





Test Results

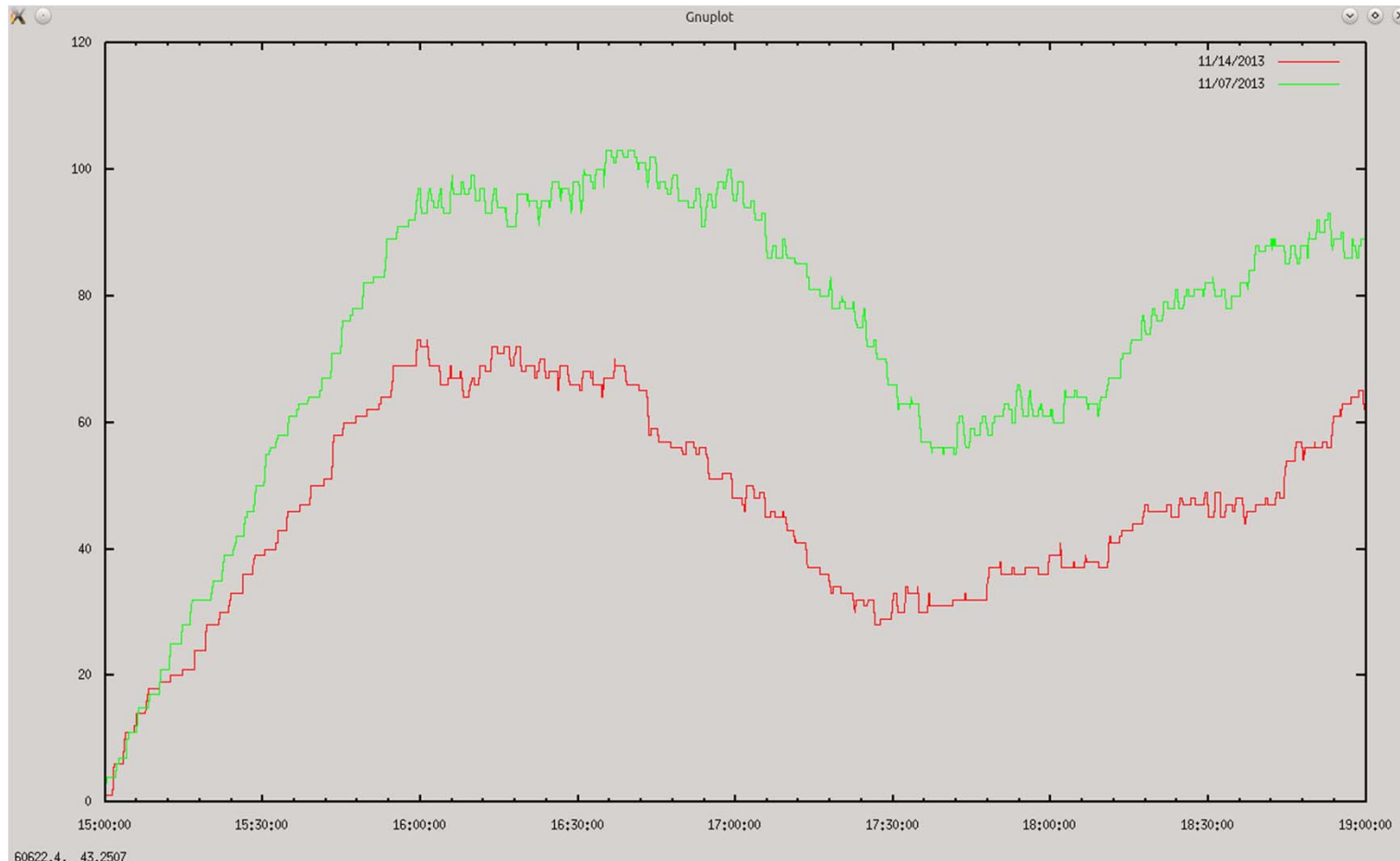
- **Data Analysis Using: Vehicle Counts & Occupancy**
 - **Taylor intersection**
 - Phase 5 (WB) LT Stop-line
 - Phase 5 (WB) LT Advance loop (A-loop)
 - Phase 6 (EB) RT A-loop
 - **Onramp**
 - Passage detector
 - Queue detector
 - **Freeway SR87 mainlines immediate upstream**
 - HOV, GPL 1, and GPL 2
 - **Aggregation**
 - Accumulated value for each day 2:15-8:00pm
 - Average of Accumulated value overall effective days



WB LT: Vehicle Count at Stop Line (Phase 5)

— After

— Before



Net Benefit at Taylor Intersection (**EB RT & WB LT**) at Advance Loop



Time interval for evaluation	4:00-6:00pm	4:00- 6:30pm	4:00-7:00pm
Total Flow of WB RT & EB LT increased	6.98%	7.64%	8.18%
Time delay reduced	7.15%	5.24%	3.06%



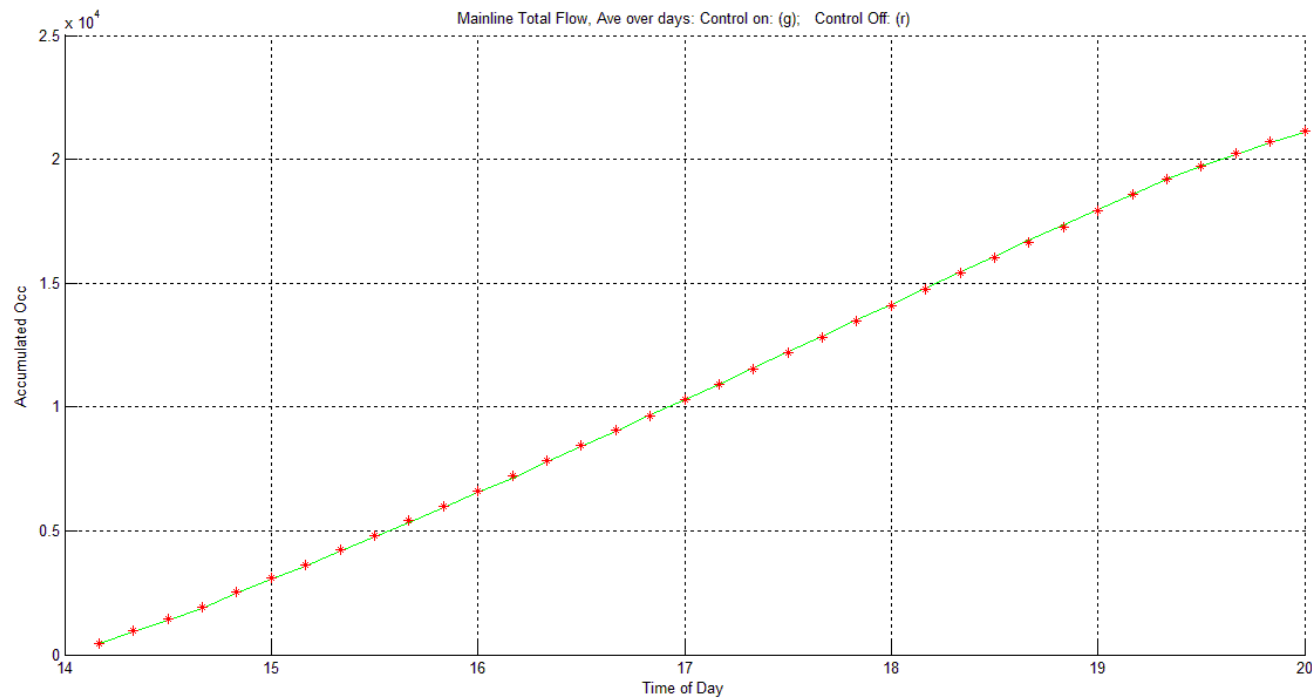


Onramp Benefit: Accumulated Flow

Time interval for evaluation	4:00-6:00pm	4:00- 6:30pm	4:00-7:00pm
Passage detector flow change (after meter)	0.73%	1.2%	1.8%
Queue detector flow change	11.01%	11.12%	11.28%



Effect on Freeway Upstream Traffic Flow



- **Freeway mainline throughput unchanged**
- **To improve freeway traffic, it is necessary to consider a freeway corridor and relevant arterial(s) in next project phase**



Performance Improvement Summary

- **Intersection**
 - **WB RT flow significantly increased by 7% in PM Peak**
 - **Total delay reduced by 7% in PM Peak**
- **Onramp**
 - **Passage detector flow increased about 1%**
 - **Queue flow increased by 11% → better use of onramp storage in PM Peak**
- **Freeway mainline upstream**
 - **End even, not affected in PM Peak**



Next Phase of the Project

- To consider a freeway corridor with relevant arterial(s)
- Coordinated ramp metering (developed in another project)
- Coordinated arterial intersection signal control
- For both freeway RM and arterial intersections
 - Centralized data systems
 - Centralized signal control
 - PATH computer interface with TMC computer
- Systematic coordination of the two subsystems
- Secured communication between two PATH computers running a special OS to avoid virus
- Objective: to achieve system-wide performance improvement





Questions and Comments?

- **Please Contact:**

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Technical Details: UP ALINEA

- Estimate downstream occupancy from upstream measurement.

$$- \tilde{o}_{out} = \alpha o_{in} \left(1 + \frac{q_r}{q_{in}}\right) \frac{\lambda_{in}}{\lambda_{out}}, \text{ if } o_{in} \leq o_{cr}$$

$$- \tilde{o}'_{out} = o_{in} \frac{\lambda_{in}}{\lambda_{out}} + 100 \frac{L}{w \lambda_{out}} q_r, \tilde{o}_{out}(k) = \gamma \tilde{o}'_{out}(k) + (1 - \gamma) \tilde{o}'_{out}(k-1), \text{ else.}$$



Technical Details: Intersection design

- EB right-turn is connected to phase 3. (phase 3 still has SB left-turn, no conflict between them.)
- Design and set max green of phase 3 every cycle, indirectly change EB right-turn green.
- Detectors associated with EB right-turn are assigned to phase 3, so they can actuate phase 3.
- Queue overwrite like design
 - **When the onramp queue is estimated to be, activate the regulation.**
 - if onramp queue or the WB left-turn queue grows, reduce max green of phase 3 by 2sec per cycle to limit the discharge of EB right-turn.
 - Otherwise, increase max green of phase 3 by 2sec to allow more vehicles to get into onramp.
 - Release EB right-turn when its queue exceeds a limit.
 - Lower bound and upper bound of the max green.
 - **Deactivate.**



Technical Details: Intersection design (cont.)

- range of the max green
 - Max green is within 15~24sec.
 - Original max extension is 15sec, min initial is 4sec.
 - Max extension of phase 7 is 20sec, min initial is 4sec.
 - By choosing 24sec, phase 3 and phase 7 are possible to terminate at the same time (yellow and red clear are different).
 - By choosing 15sec, phase 3 vehicles can be discharged (phase 3 has a very low demand).
 - Phase 3 use max green every 4 cycle to release Phase 6 RT queue



Modeling and Simulation - Model Calibration

- **Data: Aggregated over 10min**
- **Flow**
 - Compute the percentage of acceptable simulated flow
 - Aggregated flow in 10min
 - Freeway: detector at upstream of Taylor SB on-ramp, 1 lane available
 - Intersection: 8 movements
 - **Criteria**
 - **Link flow quantity**
 - 700vph < Flow < 2700vph, within 15%;
 - Flow < 700vph, within 100vph;
 - Flow > 2700vph, within 400vph;
 - **Link flow GEH**
 - $GEH = \sqrt{\frac{2(M-C)^2}{M+C}}$
- **Speed**
 - Compute the percentage of acceptable simulated speed (error<5mph)



Delay Estimation

- Assuming an average queue release speed
- Estimate queue length based on occupancy of single loop detector
 - → Determine density from flow measurement
 - → Determine the number of vehicles in the queue
 - → Accumulate per cycle to get total delay
 - → Apply to WB RT and EB LT to get total delay
 - → Apply this method to both “before” and “after” situation