

## The SWARM algorithms

## SWARM 1

- Coordinated strategy
- Has 2 primary functions: forecast and apportionment.
- Freeway is divided into sections, each section limited downstream by a bottleneck.
- Regulates density at the bottleneck section, keeping it below the *estimated saturation density*.
- Saturation density is estimated daily using a 2<sup>nd</sup> order polynomial fit on the fundamental diagram.
- Ramp metering rates are set based on forecasted bottleneck densities. Forecast is made with a Kalman filter (?)

Forecast model: Assume the current time is  $t$ . Then the basic forecast model is that density at time  $t+1$  equals the density at time  $t$  plus  $b$ , where  $b$  represents the forecasted slope of the density trend at time  $t$ . The key to the forecast is to obtain the best possible estimate of  $b$ . To accomplish this objective, two steps are required; first estimate the slope of the line in the interval  $(t-h)$  to  $t$  using a simple linear curve technique; second apply a Kalman filter (see Appendix B for test results) using the observed values of density obtained during the same time interval. The Kalman filter produces a new value of  $b$  that is then used to forecast the density at the next time period. This procedure is reiterated until  $T_{crit}$  is reached. The result is a forecasted value of density.

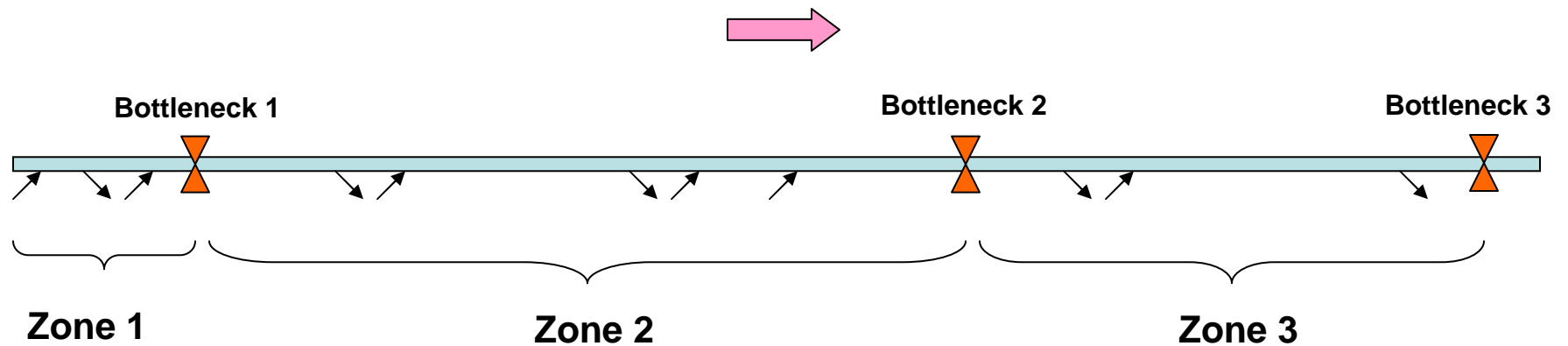
It is felt that, considering the frequency of update, the SWARM 1 algorithm will be able to anticipate the onset of congestion very effectively.

## SWARM 2

- Two local strategies (2a and 2b)
- Only one of the two can be used, perhaps in combination with SWARM 1.

SWARM 2a is theoretically based on headway theory and uses a density function to compute local metering rates. It attempts to maintain headway by optimizing density such that maximum flow can be accomplished. As such the algorithm requires no saturation densities for operation.

SWARM 2b is an algorithm based upon computing the number of vehicles stored between two VDSs. This algorithm requires measurements of volume and speed at the VDSs, tables containing distances between VDSs, and storage saturation values. These requirements are shown in the following figure depicting a Storage Zone.



- The freeway is divided at the bottlenecks into zones.
- Each zone refers to its downstream bottleneck.

- A Kalman filter is used to estimate the future bottleneck density
- The a-priori estimate is used as the “observation”!

NET code.

```
for ( i=0; i<FORECAST_LEAD_TIME; i++ ){

    SW_BOTTLENECK[k].Tcrit = SW_BOTTLENECK[k].Tcrit + 1;

    temp = SW_BOTTLENECK[k].pre_mean_density_forecast;
    SW_BOTTLENECK[k].pre_mean_density_forecast = SW_BOTTLENECK[k].post_mean_density_forecast * MODEL_PARA_A
        + SW_BOTTLENECK[k].norm_station_density_slope_forecast * MODEL_PARA_B;

    SW_BOTTLENECK[k].pre_variance_density_forecast =
        pow( MODEL_PARA_A, (double)2 ) * SW_BOTTLENECK[k].post_variance_density_forecast
        + pow( MODEL_PARA_G, (double)2 ) * MODEL_PARA_Q;

    observation_station_density = SW_BOTTLENECK[k].pre_mean_density_forecast;

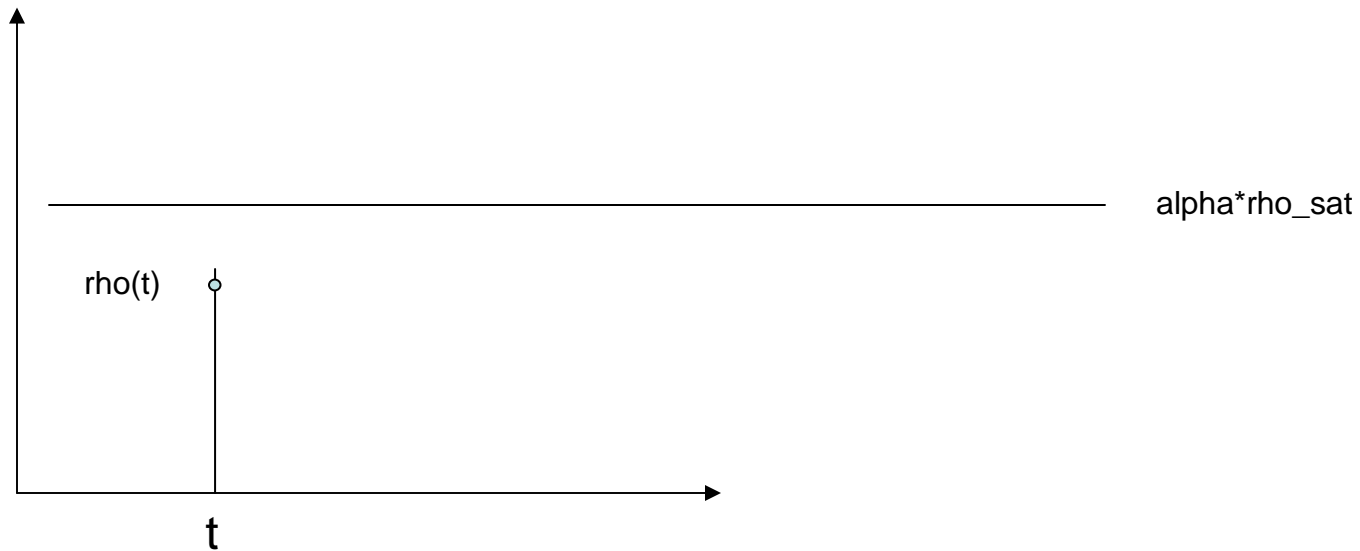
    SW_BOTTLENECK[k].post_variance_density_forecast = 1/( 1/ SW_BOTTLENECK[k].pre_variance_density_forecast
        + MODEL_PARA_H * MODEL_PARA_H/MODEL_PARA_R );

    SW_BOTTLENECK[k].post_mean_density_forecast = SW_BOTTLENECK[k].pre_mean_density_forecast
        + SW_BOTTLENECK[k].post_variance_density_forecast * ( MODEL_PARA_H/MODEL_PARA_R )
        * ( observation_station_density - MODEL_PARA_H * SW_BOTTLENECK[k].pre_mean_density_forecast );

    for ( j=0; j < SLOPE_SAMPLE_SIZE - 1; j++ )
        SW_BOTTLENECK[k].slope_density_forecast[j] = SW_BOTTLENECK[k].slope_density_forecast[j+1];
        SW_BOTTLENECK[k].slope_density_forecast[SLOPE_SAMPLE_SIZE-1] = observation_station_density;

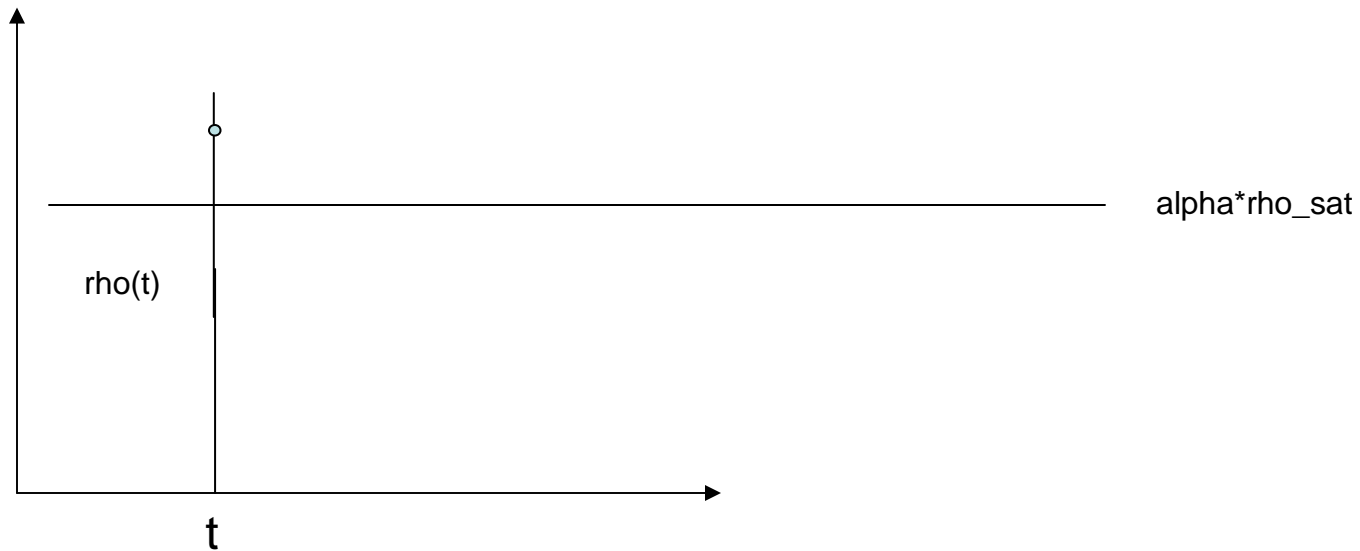
    SW_BOTTLENECK[k].norm_station_density_slope_forecast = density_slope_calculator_forecast(k);

}
```



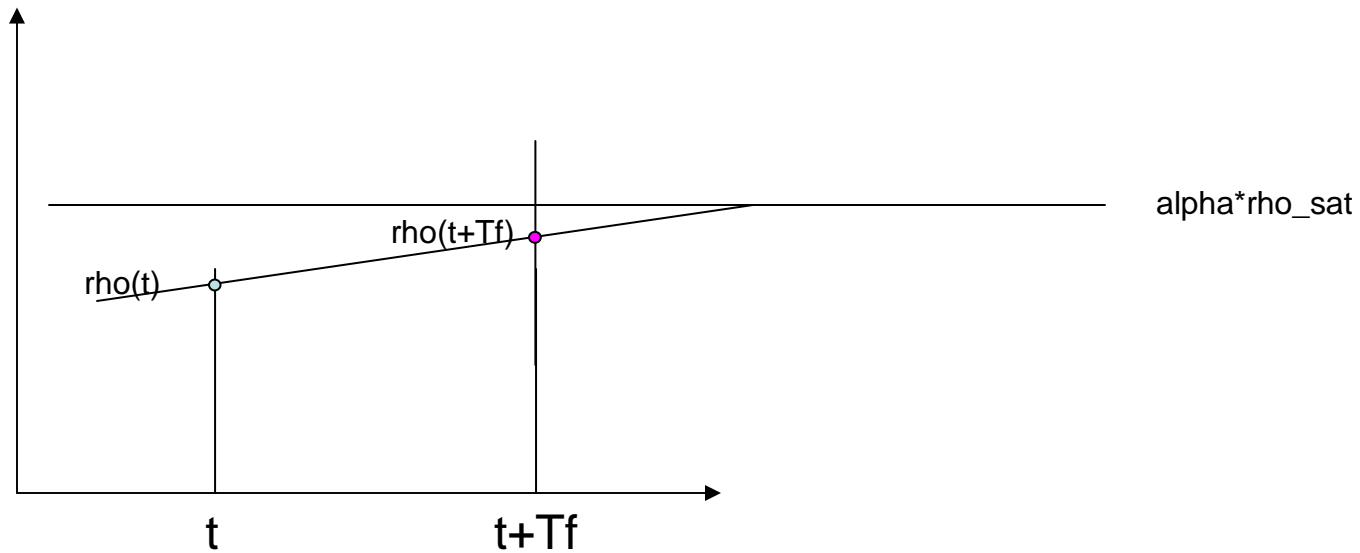
For each zone, at time  $t$  we check whether the measured density at the bottleneck is above or below  $\alpha \cdot \rho_{\text{sat}}$ , where  $\rho_{\text{sat}}$  is the critical density and  $\alpha < 1$ .

We want to calculate  $\rho_{\text{req}}(t)$ , the current density required so that the bottleneck will not become saturated  $T_f$  seconds in the future.



If  $\rho(t) > \alpha \cdot \rho_{\text{sat}}$   $\rightarrow \rho_{\text{req}}(t) = \alpha \cdot \rho_{\text{sat}}$

The current value of density must be less than saturation density to proceed with a forecast. If the current density is greater than or equal to saturation density, the forecasting portion of the algorithm is skipped.

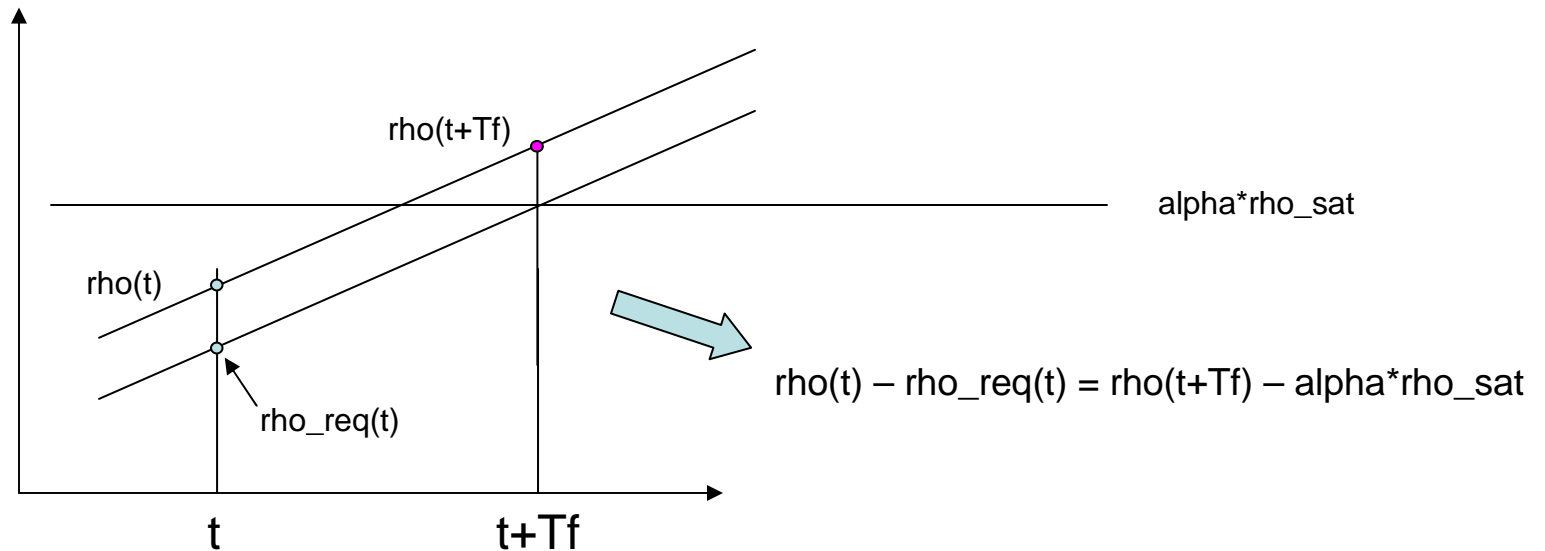


If  $\rho(t) > \alpha \cdot \rho_{\text{sat}}$   $\rightarrow \rho_{\text{req}}(t) = \alpha \cdot \rho_{\text{sat}}$

Otherwise estimate  $\rho(t+T_f)$

If  $\rho(t+T_f) < \alpha \cdot \rho_{\text{sat}}$   $\rightarrow \rho_{\text{req}}(t) = \alpha \cdot \rho_{\text{sat}}$

If the forecasted density is less than saturation density or the current density is greater than or equal to saturation density then the required bottleneck density equals saturation density.



If  $\rho(t) > \alpha \cdot \rho_{sat}$   $\rightarrow \rho_{req}(t) = \alpha \cdot \rho_{sat}$

Otherwise estimate  $\rho(t+T_f)$

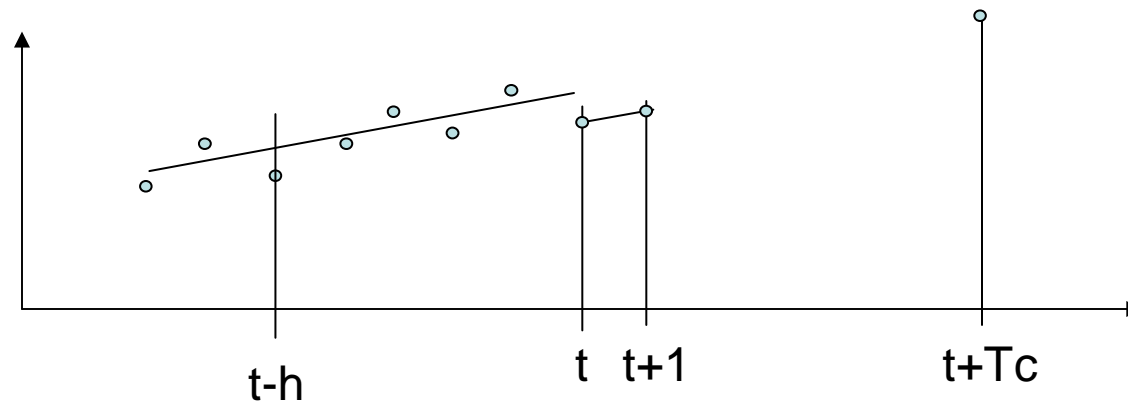
If  $\rho(t+T_f) < \alpha \cdot \rho_{sat}$   $\rightarrow \rho_{req}(t) = \alpha \cdot \rho_{sat}$

Otherwise

$$\rho_{req}(t) = \rho(t) - \rho(t+T_f) - \alpha \cdot \rho_{sat}$$

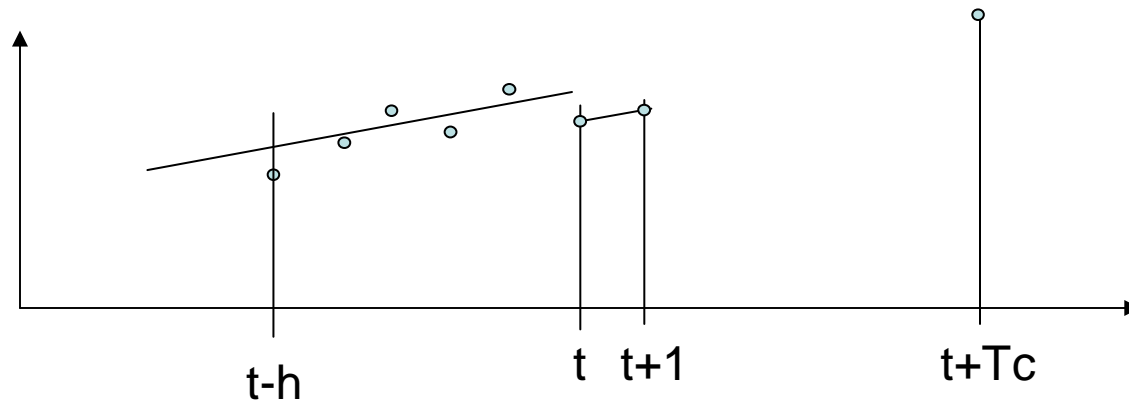
If the forecasted value of density exceeds the bottleneck's saturation density then the required bottleneck density will be computed as follows: current density minus  $1/T_{crit}$  times the amount of excess density (above saturation density) computed above.

units?



Forecast model: Assume the current time is  $t$ . Then the basic forecast model is that density at time  $t+1$  equals the density at time  $t$  plus  $b$ , where  $b$  represents the forecasted slope of the density trend at time  $t$ . The key to the forecast is to obtain the best possible estimate of  $b$ . To accomplish this objective, two steps are required; first estimate the slope of the line in the interval  $(t-h)$  to  $t$  using a simple linear curve technique; second apply a Kalman filter (see Appendix B for test results) using the observed values of density obtained during the same time interval. The Kalman filter produces a new value of  $b$  that is then used to forecast the density at the next time period. This procedure is reiterated until  $T_{crit}$  is reached. The result is a forecasted value of density.

It is felt that, considering the frequency of update, the SWARM 1 algorithm will be able to anticipate the onset of congestion very effectively.



•  $b =$  least squares on data in  $[t-h\dots t]$  : 
$$b = \frac{h \sum k x_k - \sum k \sum x_k}{h \sum k^2 - (\sum k)^2}$$

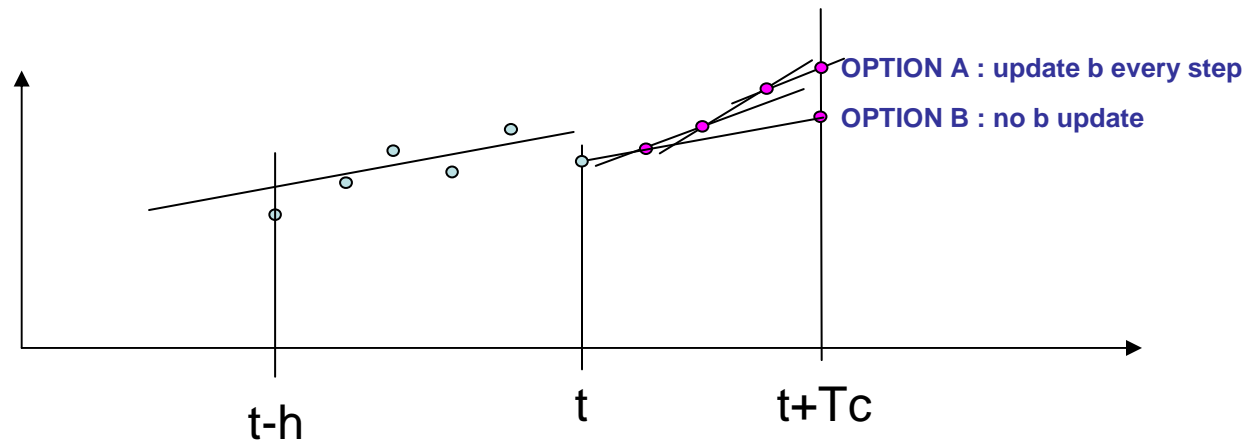
• Model: 
$$\begin{aligned} x_{k+1} &= x_k + b + w_k \\ y_k &= x_k + v_k \end{aligned}$$

• “Forecast” densities in  $[t-h\dots t]$  using Kalman filter:

$$\left\{ \begin{aligned} \hat{x}_k^- &= \hat{x}_{k-1} + b \\ \hat{P}_k^- &= \hat{P}_{k-1} + Q \\ K_k &= \hat{P}_k^- / (\hat{P}_k^- + R) \\ \hat{P}_k &= (1 - K_k) \hat{P}_k^- \\ \hat{x}_k &= \hat{x}_k^- + K_k (z_k - \hat{x}_k^-) \end{aligned} \right.$$

• Forecast densities in  $[t-h\dots t]$  using “Kalman filter”:

$$\left\{ \begin{aligned} \hat{x}_k^- &= \hat{x}_{k-1} + b \\ \hat{P}_k^- &= \hat{P}_{k-1} + Q \\ K_k &= \hat{P}_k^- / (\hat{P}_k^- + R) \\ \hat{P}_k &= (1 - K_k) \hat{P}_k^- \\ \hat{x}_k &= \hat{x}_k^- + K_k (z_k - \hat{x}_k^-) \end{aligned} \right. \quad z_k = \hat{x}_k^- \quad \text{!}$$

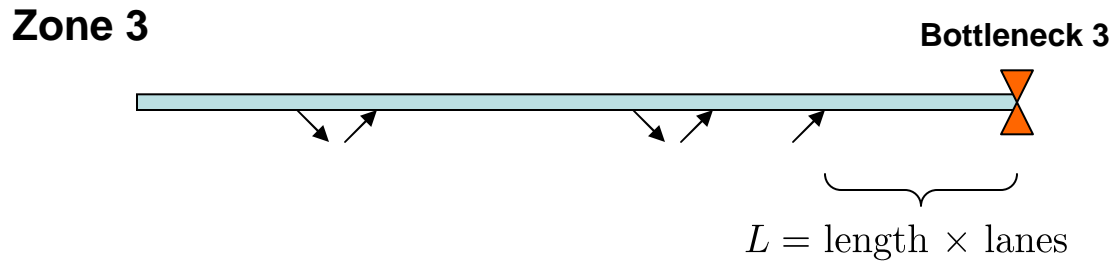


Once we have calculated  $\rho_{req}(t)$  for each zone, we need to find the metering rates that will bring the current density at the bottleneck to  $\rho_{req}$ .

We do this by converting the density difference into an adjustment to the metering rate, and then apportioning this adjustment to the different ramps within the zone.

The algorithm involves:

- rex ... excess onramp metering rate
- rdes .... Desired onramp metering rate
- rc ... actual onramp metering rate



$$r_{ex} = 0$$

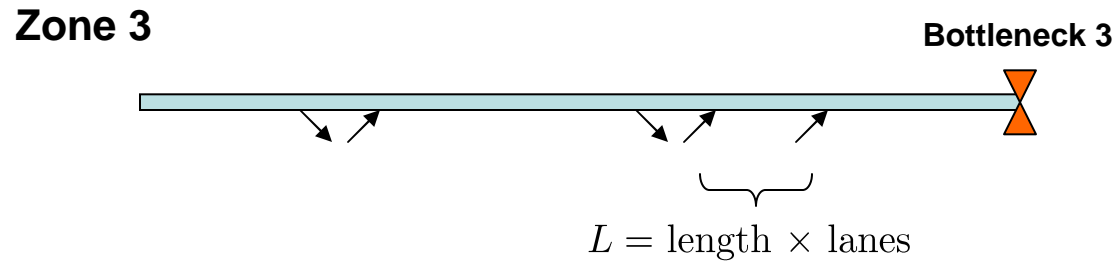
$$\Delta r = L(\rho_b(t) - \rho_{req})$$

$$r_{des} = r(t) - \Delta r - r_{ex}$$

$$r_c = \min(\max(r_{des}, r_{min}), r_{max})$$

$$r_{ex} = \phi(r_c - r_{des})$$

- $\phi$  is an intra-zone propagation factor.
- $r_{ex}$  can be positive or negative.

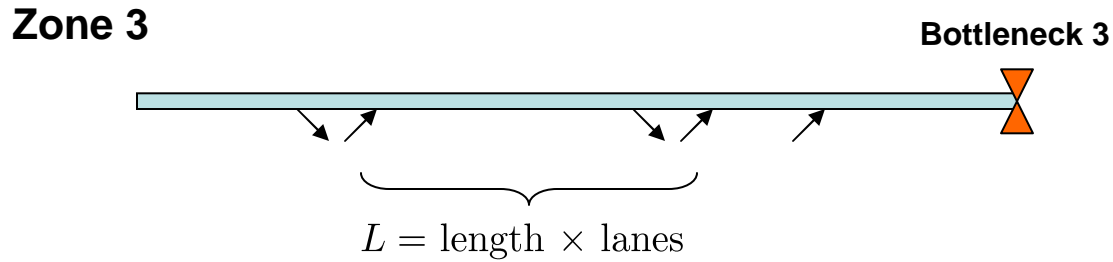


$$\Delta r = L(\rho_b(t) - \rho_{req})$$

$$r_{des} = r(t) - \Delta r - r_{ex}$$

$$r_c = \min(\max(r_{des}, r_{min}), r_{max})$$

$$r_{ex} = \phi(r_c - r_{des})$$



$$\Delta r = L(\rho_b(t) - \rho_{req})$$

$$r_{des} = r(t) - \Delta r - r_{ex}$$

$$r_c = \min(\max(r_{des}, r_{min}), r_{max})$$

$$r_{ex} = \psi\phi(r_c - r_{des})$$

- $\psi$  is an inter-zone propagation factor.

Given

$j$  = VDS upstream of ramp station number (numbered upstream)

$t$  = Sample time interval

$n_j$  = Number of Lanes at VDS <sub>$j$</sub>

$D_{j,t}$  = Density at VDS <sub>$j$</sub>  at time  $t$  (Veh/Mile/Lane)

$S_{j,t}$  = Station Speed at VDS <sub>$j$</sub>  at time  $t$  (Miles/Hour)

$\lambda_j$  = Vehicle length at VDS <sub>$j$</sub>  (feet)

$\rho_j$  = Postmile at VDS <sub>$j$</sub>  (miles)

$\xi_j$  = Target speed reduction (Feet/Sec); initial value = 1

$Rmax_j$  = Maximum metering rate at ramp  $j$  (Vehicles per minute)

$Rmin_j$  = Minimum metering rate at ramp  $j$  (Vehicles per minute)

Then the following is computed for each ramp at time  $t$

$$\text{Speed in } \frac{\text{ft}}{\text{sec}} \quad C_{j,t} = S_{j,t} * \frac{5280}{3600} \text{ (Feet/Sec)}$$

$$\text{Headway at VDS}_j \quad H_{j,t} = \frac{5280 - D_{j,t} * \lambda_j}{D_{j,t} * C_{j,t}} \text{ (seconds)}$$

$$\text{Speed Reduction per mile} \quad \Delta s = \frac{\xi_j}{\rho_j - \rho_{j-1}}$$

$$\text{Then Compute} \quad \tau_{j,t} = 19 - 2 * n_j * (\rho_j - \rho_{j+1}) \left[ \frac{5280}{(C_{j,t} - \Delta s) * H_{j,t} + \lambda_k} - D_{j,t} \right]$$

$$\text{Metering Rate} \quad R_{j,t} = \text{Minimum}(\text{Maximum}(\tau_{j,t}, Rmin_j), Rmax_j)$$



Given

$j$  = First Good VDS upstream of metered ramp  $j$

$j+1$  = First Good VDS downstream of metered ramp  $j$

Zone  $j$  = The section of freeway between  $VDS_j$  and  $VDS_{j+1}$

$n_j$  = Number of Lanes at  $VDS_j$

$V_{j,t}$  = Volume at  $VDS_j$  at time  $t$

$SumE_{j,t}$  = Sum of Volumes at all entrance ramps between  $VDS_j$  and  $VDS_{j+1}$  at time  $t$

$SumEx_{j,t}$  = Sum of Volumes at all exit ramps between  $VDS_j$  and  $VDS_{j+1}$  at time  $t$

$w_j$  = Weighing factor for final Storage computation (Initial value = 0.1)

$D_{j,t}$  = Density at  $VDS_j$  at time  $t$  (Veh/Mile/Lane)

$\epsilon^{e-d}$  = Percentage reduction in density LOS E to LOS D

$Dsat_j$  = Saturation Density at  $VDS_j$  at time  $t$  (Veh/Mile/Lane)

$\rho_j$  = Postmile at  $VDS_j$  (miles)

$Rmax_j$  = Maximum metering rate at ramp  $j$  (Vehicles per minute)

$Rmin_j$  = Minimum metering rate at ramp  $j$  (Vehicles per minute)

Then the following is computed for each ramp at time t

$$\text{Estimated Storage in Zone } j \quad \hat{\sigma}_{j,t} = \frac{n_j (\rho_j - \rho_{j+1})(D_{j,t} + D_{j+1,t})}{2}$$

$$\text{Net Change in Zone } j \quad \Delta_t = V_{j,t} - V_{j+1,t} + \text{Sum}E_{j,t} - \text{Sum}E_{j+1,t}$$

$$\text{Cumulative Storage in Zone } j \quad \bar{\sigma}_{j,t} = \sigma_{j,t-1} + \Delta_t \quad (\sigma_{j,t-1} \text{ defined below})$$

IF any ramp detectors are failed in Zone j THEN

$$k = 1$$

ELSE

$$k = w_j$$

ENDIF

$$\text{Storage in Zone } j \quad \sigma_{j,t} = k * \hat{\sigma}_{j,t} + (1-k) * \bar{\sigma}_{j,t}$$

$$\text{Critical Storage in Zone } j \quad \sigma_{j,t}^{crit} = \frac{n_j (\rho_j - \rho_{j+1})(D_{sat,j} + D_{sat,j+1})}{2}$$

$$\text{Max Available Capacity in Zone } j \quad C_{j,t}^{max} = \sigma_{j,t}^{crit} - \sigma_{j,t}$$

$$\text{Desired Available Capacity in Zone } j \quad C_{j,t}^{des} = C_{j,t}^{max} * e^{-d}$$

IF  $C_{j,t}^{des} > Rmin_j$  THEN

$$\text{Metering Rate } R_{j,t} = \text{Minimum}(C_{j,t}^{des}, Rmax_j)$$

ELSE

$$\text{Metering Rate } R_{j,t} = \text{Maximum}(\text{Minimum}(C_{j,t}^{max}, Rmax_j), Rmin_j)$$

ENDIF